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### VULNERABILITY OF DIKES OR GRAVITY DAMS Better estimate the strength of rockfills

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## OUTLINE

- Rockfill ?
- Brief history of dikes and rockfill dams
- Geotechnical investigation
  - Mechanical part
  - Hydraulic part
- Erosion



### **Rockfill ?**



- Collection of rock particles : purely frictional, non cohesive
- From some mm to meter in civil engineering applications
- Usually placed in dense conditions









river dikes

coastal dikes

railway

dam

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PERIOD 1 – From ancient times (> 2000 years) to Middle Age

Agriculture, flooding mitigation, water ressource

Small and massive dams (< 25m)



Figure 1 : Barrage de SADD-EL-KAFARA (-2650 av JC) d'après SCHNITTER

Engineering point of view : double need of STABILITY / WATERTIGTHNESS

PERIOD 2 – Industrial revolution (19th – beginning of 20th century)

Technical improvements : concrete, arch technology (thinner dams)



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« Zola » dam (France)  $_4$ 

PERIOD 3 – Post 2<sup>nd</sup> world (1950 – 1970)

Higher and higher structures (> 250m)

Objectives : energy production, irrigation

#### **2** main catastrophic events on concrete arch dams

Location : Europe / North America (USA, Mexico)

Malpasset (France), 1959,

*Quality of the abutment* 



Landslide in the reservoir





**PERIOD 4 - > 2000** 

Main areas of interest : South America, Africa & Asia

Main technology:

CFRD (Concrete Face Rockfill Dam)

Heigths : > 200m



Genesis of SUSTAINABLE DEVELOPMENT : main consequence = RE-USE of LOCAL MATERIAL

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PERIOD 4 -> 2000

PATHOLOGIES

CASE STUDY 1 / CAMPOS NOVOS DAM (BRAZIL - 2005)

Hydro-power, H 202m

FAILURE of the concrete face during 1st filling





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PERIOD 4 -> 2000

PATHOLOGIES

CASE STUDY 2 / MOHALE DAM (LESOTHO - 2006)

H 145m,

FAILURE of the concrete face during 1st filling

+ HEAVY RAINS



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Leakage of 600 l/s

**PERIOD 4 - > 2000** 

PATHOLOGIES

CFRD	ISSUE	CAUSE
Aguamilpa h=187m	Concrete facing cracking	Rockfill deformability
Barra Grande h=185m	Concrete facing cracking	Joint failures
Campos Novos h=202m	Concrete facing cracking	Rockfill defformability
ltá h=125m	Slabs cracking	Rockfill deformability
ltapebi h=120m	Cracks parallel to the plinth	Foundation geometry
Mohale h=145m	Compression joint rupture	Rockfill deformability
Tianshengqiao 1 h=178m	Horizontal cracking	Construction sequence
Xingó h=150m	Slabs cracking	Sharp geometry of the left abutment and material deformability

 $\triangleright$ 

PERIOD 4 -> 2000

ACCIDENTOLOGY

#### SIMILARITIES

- PERIOD : 2000 < < 2010
- CFRD involving ROCKFILL
- FAILURE during 1st FILLING
- FAILURE of the CONCRETE face
  - Use of LOCAL resources

#### **COMMON FEATURES**

- No technical capacity to evaluate rockfill quality
  - High mechanical stresses
  - Lack of geotechnical knowledge
    - Influence of water

#### **MECHANICAL / HYDRO ISSUE**

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**MECH** ISSUE 1: ANY SIZE EFFECT AT THE « GRAIN » SCALE ?



Protodyakonov tests Franklin tests

•••

 $\sigma_f \propto \frac{F_f}{d^2}$ 



#### YES THERE IS A SCALE EFFECT AT THE GRAIN SCALE

(Weibull 's distribution)



Marsal et al. (1965) El Infiernillo - Diorite
× Marsal et al. (1965) Pinzadaran: Gravelly alluvial
\* Marsal (1967) Mica grad. Y - Granitic gneiss
+ Marsal (1967) San Francisco grad. 2- Basalt
Trois Vallées ECHO - Shale Rockfill Entre Plaques
Trois Vallées ECHO - Shale Rockfill Franklin
A Calcaire de Préfontaines - Entre Plaques ECN

11

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MECH ISSUE 2 : IT THE SAME AT THE SAMPLE SCALE ?

SAMPLE SIZE : at least 6 to 10 times the maximum grain size











#### Extrapolation to rockfill in the field

**MECH** ISSUE 3 : IT THE SAME AT THE SAMPLE SCALE ?



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#### **CONCLUSION: 3**

#### YES

- Decrease of the shear strength with increasing particle size
  - Increase of grain breakage
- More compressible volumetric behaviour (more prone to settlement)

## **HYDR**







SETTLEMENT EMPHASIZED BY RAINFALLS

rate



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1D compression tests

**Clear instanteneous effect of imbibition** 





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1D compression tests

Long-term effect in both dry or saturated rockfills





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1D compression tests

Long-term effect in both dry or saturated rockfills



**CAMPOS NOVOS CFRD** 

- 1st filling of the reservoir

Contact between water and rock blocks

U Breakage of blocks at the base

↓ Settlement of the rockfill + bending of the concrete face (traction)

↓ Failure of the concrete face + leakage





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## **Erosion**



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#### **External erosion (backward)**

#### Detachment of particles downstream

↓ Loss of stability

#### **Internal erosion**

Flow of small particles inside the structure

Change of mechanical and hydraulic properties that could lead to failure

## Conclusions

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- Rockfill behaviour not completely known
- Use of local rock ressources : not mechanically optimized
- Material senstive to stresses and water (imbibition or flow)
- Time-dependency of the behaviour : requirement of an adequate monitoring of the structures

### THANK YOU FOR YOUR ATTENTION

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